

Crack width limitation acc. to DIN 1045-1: 2008-08**Requirements**

Permissible crack width $w_k = 0,15$ mm.

Materials, geometry

Rectangular cross section with height = 80,0 cm; width = 100,0 cm

Concrete quality: C20/25

Concrete cover c_{nom} : 3,5 cm

Bar diameter: $d_{s,exist.} = 16,0$ mm (Reinforcing steel)

Check of the minimum reinforcement

Action: Central tension

A_s is computed for one layer \Rightarrow Tensile zone height = component height / 2

Location of restraint: inside of the component

Concrete age at time of cracking: 6 to 28 days

Height of the effective area of the reinforcement as per Pict. 53 d): $h_{eff} = 16,60$ cm

Section 11.2.2 (8): For thick components under central tension, the minimum reinforcement can be calculated considering the effective edge zone $A_{c,eff}$ where $A_{c,eff} = h_{eff} \cdot b$ and $A_{ct} = 0.5 h \cdot b$.

$$A_{s,min} = f_{ct,eff} \cdot A_{c,eff} / \sigma_s \geq k \cdot f_{ct,eff} \cdot A_{ct} / f_{yk} \quad (130 a,b)$$

$$\begin{array}{llll} \text{with } k = 0,50 & f_{ct,eff} = 2,20 \text{ N/mm}^2 & A_{c,eff} = 1075,0 \text{ cm}^2 & \\ A_{ct} = 4000,0 \text{ cm}^2 & f_{yk} = 500 \text{ N/mm}^2 & \sigma_s = \text{see following calculation} & \end{array}$$

The limiting of crack width can be proved by limiting of the bar diameter.

$$\begin{aligned} \phi_s &= \phi_s^* \cdot f_{ct,eff} / 3.0 \\ \Rightarrow \phi_s &= \phi_s^* \cdot 0,73 \quad \Rightarrow \phi_s^* = 16,0 \text{ mm} / 0,73 = 21,82 \text{ mm} \\ \Rightarrow \sigma_s &= 160,00 \text{ N/mm}^2 \text{ as per Table 20} \\ \Rightarrow A_{s,min} &= 22,83 \text{ cm}^2 \geq 8,80 \text{ cm}^2 \end{aligned}$$

$$A_{s,min} = 22,83 \text{ cm}^2 \text{ per layer}$$

Check of crack width limitation by direct calculation

Concrete age at time of cracking: > 28 days

$A_{s,exist.} = 24,00 \text{ cm}^2$ in the layer to be proved

Steel stress $\sigma_s = 180,00 \text{ N/mm}^2$

Component: Beam

Difference of the average strain of concrete and reinforcement:

$$\varepsilon_{sm} - \varepsilon_{cm} = \frac{\sigma_s - 0,4 \cdot \frac{f_{ct,eff}}{eff\rho} \cdot (1 + \alpha_e \cdot eff\rho)}{E_s} \geq 0,6 \cdot \frac{\sigma_s}{E_s} \quad (136)$$

$$\begin{array}{lll} \text{with } eff\rho = 0,0223 & E_s = 200000 \text{ N/mm}^2 & E_{cm} = 24900 \text{ N/mm}^2 \\ f_{ct,eff} = 2,20 \text{ N/mm}^2 & \alpha_e = E_s / E_{cm} = 8,0 & \end{array}$$

$$\varepsilon_{sm} - \varepsilon_{cm} = 0,00067 \geq 0,00054 \Rightarrow \varepsilon_{sm} - \varepsilon_{cm} = 0,00067$$

Maximum crack distance:

$$s_{r,max} = \frac{d_s}{3,6 \cdot \text{eff}\rho} \leq \frac{\sigma_s \cdot d_s}{3,6 \cdot f_{ct,eff}} \quad (137)$$

$$s_{r,max} = 199,1 \leq 363,6 \Rightarrow s_{r,max} = 199,1 \text{ mm}$$

Crack width:

$$w_k = s_{r,max} \cdot (\varepsilon_{sm} - \varepsilon_{cm}) \quad (135)$$

$$w_k = 0,13 \text{ mm} \leq w_{k,zul} = 0,15 \text{ mm} \quad \text{The check is OK!}$$